

《原著》

Applications of Diverse Data Combinations in Subsurface Characterization using D-optimality Based Pilot Point Methods (DBM)

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ABSTRACT

Many cases of strategically designed groundwater remediation have lack of information of hydraulic conductivity or permeability, which can render remediation methods inefficient. Many studies have been carried out to minimize this shortcoming by determining detailed hydraulic information either through direct or indirect measurements. One popular method for hydraulic characterization is the pilot point method (PPM), where the hydraulic property is estimated at a small number of strategically selected points using secondary measurements such as hydraulic head or tracer concentration. This paper adopted a D-optimality based pilot point method (DBM) developed previously for hydraulic head measurements and extended it to include both hydraulic head and tracer measurements. Based on different combinations of trials, our analysis showed that DBM performs well when hydraulic head is used for pilot point selection and both hydraulic head and tracer measurements are used for determining the conductivity values.

Key words : Subsurface characteristics, Pilot point methods, D-optimality, Diverse data sets

1. Introduction

Pilot point method (PPM) is a well-developed inverse method designed to estimate hydrogeologic heterogeneity under either deterministic or stochastic conditions while honoring the existing hydraulic conductivity measurements. De Marsily et al. (1984) introduced this method to increase the ability of heterogeneity recognition with minimizing uncertainty. While this method decreased some of the drawbacks of other inverse methods such as over parameterization considerable degree of non-uniqueness and uncertainty still remained. Researchers have attempted to reduce non-uniqueness and uncertainty in various aspects of PPM. For instance, plausibility terms defined as boundaries of search spaces or solution smoothness, have been used in PPM (LaVenue et al., 1995; RamaRao et al., 1995; Gomez-Hernandez et al., 1997; Kowalsky et al., 2012).

The quality and quantity of observations can significantly reduce the model error and parameter uncertainty.

For the quality improvement of measured data, sensitivity techniques have been used to minimize the effects of the observed secondary information errors because small changes of hydraulic conductivities at sensitive pilot point locations highly affect the secondary information changes and vice versa. LaVenue and Pickens (1992) first adopted this sensitivity terms in pilot point location selection and it is continually used by other researchers (LaVenue et al., 1995; RamaRao et al., 1995). Jung et al. (2011) adopted D-optimality criteria to search a highly sensitive collection of pilot points with respect to the quality improvement of measured data. As a way to improve the quantity of observation data to reduce parameter errors and model non-uniqueness, “coupled inverse models” have garnered more attention. In the context of groundwater inverse problems, the term coupled inverse models is typically referred to problems where both flow and transport parameters are estimated simultaneously or both flow and transport measurements are involved in estimating either flow or transport parameters

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원고접수일 : 2013. 4. 9 심사일 : 2013. 4. 22 게재승인일 : 2013. 4. 22
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(Sun and Yeh, 1990). The latter definition is considered in this study. In this context, coupled inverse models increase the measured information beside hydraulic head values, such as steady state temperatures (Woodbury et al., 1987; Woodbury and Smith, 1988), mass transport (Strecker and Chu, 1986), solute arrival time quantile (Harvey and Gorelick, 1995). While coupled inverse problems has been investigated extensively in groundwater modeling (Strecker and Chu, 1986; Mishra and Parker, 1989; Median et al., 1990; Wen et al., 2002; Franssen et al., 2003; Liang et al., 2010), we are not aware of any work in the context of pilot point methods.

The objectives of this study are to (1) extend a previously developed D-optimality based PPM method to utilize tracer measurements in addition to hydraulic head measurements, 2) evaluate its performance for a simple 2-dimensional test problem to estimate hydraulic conductivity.

2. Forward Simulation Model

The pilot point inverse modeling procedure adopted in this paper requires repeatedly executing a forward groundwater model to obtain hydraulic heads and tracer concentrations. It is assumed that the groundwater flow is steady and the tracer is conservative. The domain is assumed to be two-dimensional. The governing equations describing steady state saturated groundwater flow and conservative transport are given by equations (1) and (2):

$$\frac{\partial}{\partial x}\left(Kb\frac{\partial h}{\partial x}\right) + \frac{\partial}{\partial y}\left(Kb\frac{\partial h}{\partial y}\right) = 0 \quad (1)$$

$$\frac{\partial C}{\partial t} = \frac{\partial}{\partial x}\left(D_L\frac{\partial C}{\partial x}\right) + \frac{\partial}{\partial y}\left(D_{TH}\frac{\partial C}{\partial y}\right) - \frac{\partial}{\partial x}(V_x C) - \frac{\partial}{\partial y}(V_y C) \quad (2)$$

Where K is the hydraulic conductivity with b , thickness of confined aquifer, and h is the hydraulic head. V_x and V_y are flow velocities in the x and y directions that are computed using Darcy's law from the hydraulic heads h obtained from equation (1). D_L and D_{TH} are dispersion coefficients in the longitudinal and transverse horizontal directions that are dependent on V_x and V_y . Equations (1) and (2) are discretized using central finite differences and solved in a MATLAB environment.

3. Calibration: D-optimality Based Method (DBM)

The D-optimality based method (DBM) for hydraulic conductivity characterization using hydraulic head measurements has been described in greater detail in Jung et al., 2011. Here we provide a brief overview. DBM involves two major steps: (1) search for a set of pilot point locations that are both sensitive and uncorrelated by maximizing the D-optimality metric (to be described later), (2) search for optimal hydraulic conductivity values at these locations so as to minimize the difference between the observed and calculated measurements. For these two procedures, optimization problems are formulated as a maximization of the first objective (D-optimality - equation (3)) and minimization of the second objective (sum of squared errors - equation (6)). For optimizing these objectives, genetic algorithm is adopted as a global search method.

The DBM approach used in this study follows the basic PPM approach outlined by De Marsily et al. (1984). First, an initial hydraulic conductivity field is obtained from existing hydraulic conductivity measurements using kriging; second, select pilot point locations where hydraulic conductivity values will be estimated; and, third, iteratively run forward models (flow and/or transport) to find optimal hydraulic conductivity values at the pilot point locations such that the resulting kriged field minimizes the discrepancy between calculated and observed hydraulic heads and/or transport concentrations. A finalized hydraulic conductivity distribution will be obtained when the objective function meets the stopping criteria in automated calibration. The stopping criteria of DBM in a Genetic Algorithm application are the consecutive number of generations without objective function improvement, the installed stop time with no changes of objective, and/or total time limit for whole generations.

3.1. Objective Functions

In DBM, two sequentially separated procedures are performed based on two different optimization criteria as shown in the flow chart (Fig. 1). First objective is maximization of determinant of squared sensitivity matrix while determining pilot point locations, and the second objective

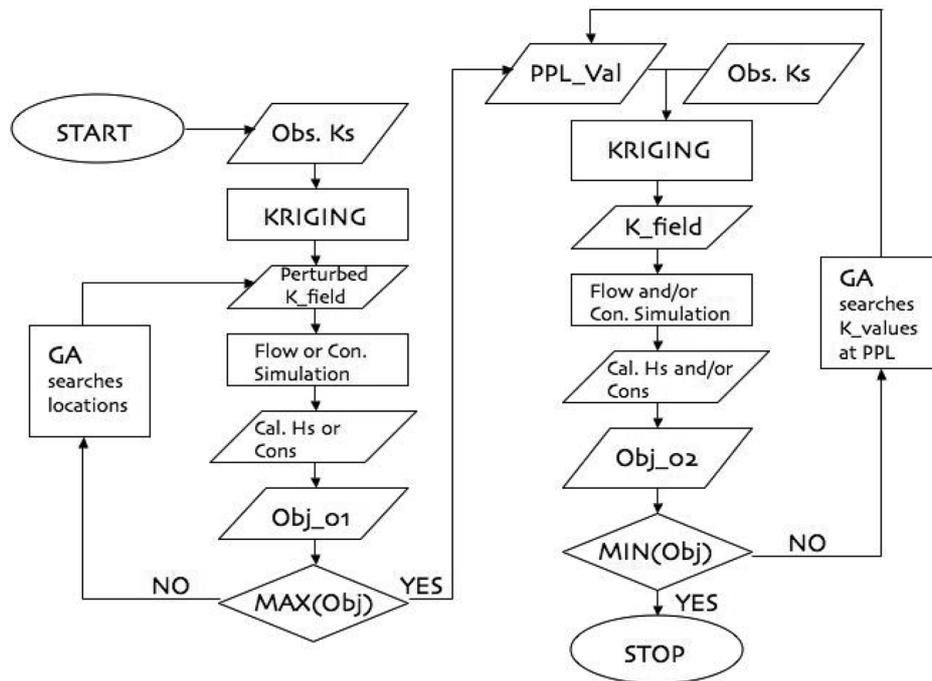


Fig. 1. Flow chart of coupled inverse problems using DBM.

for the determination of hydraulic conductivities at pilot point locations is the sum of squared residuals of observed and calculated secondary information such as hydraulic heads, tracer concentration, and/or tracer arrival quantile. Explanations in detail of objective functions are as shown below.

As the first objective function, D-optimality consists of a sensitivity matrix and determinant function as presented in equations (3) and (4). The maximum of the determinant of the Fisher information ($X^T X$), which is proportional to the inverse of covariance matrix, is equivalent to minimizing the norm of covariance matrix (Knopman et al., 1987). By maximizing D-optimality, we are achieving the following two desired goals: (1) selected pilot points are highly sensitive to the secondary information (thus less susceptible to measurement errors), and (2) the selected pilot points are less correlated to each other (less redundancy). In other words, small perturbations of the parameter (i.e., hydraulic conductivity) at selected pilot points will be sufficient to match calculated values to the measurements of secondary information (i.e., hydraulic heads and tracer concentrations). In addition, perturbations in hydraulic conductivity at any two points from this set will not produce similar changes in secondary information. In this study hydraulic

heads, tracer concentrations, and arrival time quantiles of cumulated concentrations (Harvery and Gorelick, 1995) are used as measurement quantities to search for the pilot point locations. The concentration data and quantiles generally have more data points than steady state hydraulic head measurements, since they are dependent on time (e.g. every 10 days measurements) and selected quantile times (e.g. 0.25, 0.5, and 0.75). The formulations of first objective function, D-optimality including various data sensitivity matrixes, and tracer arrival time quantiles are presented as

$$MAXIMIZE \text{obj}_1 = [X^T X] \tag{3}$$

$$X_{(n \times m)} = \begin{bmatrix} \frac{\partial h_1}{\partial k_1} & \frac{\partial h_1}{\partial k_2} & \dots & \frac{\partial h_1}{\partial k_m} \\ \frac{\partial h_2}{\partial k_1} & \frac{\partial h_2}{\partial k_2} & \dots & \frac{\partial h_2}{\partial k_m} \\ \dots & \dots & \dots & \dots \\ \frac{\partial h_n}{\partial k_1} & \frac{\partial h_n}{\partial k_2} & \dots & \frac{\partial h_n}{\partial k_m} \end{bmatrix}, X_{(nt \times m)} = \begin{bmatrix} \frac{\partial C_{1t}}{\partial k_1} & \frac{\partial C_{1t}}{\partial k_2} & \dots & \frac{\partial C_{1t}}{\partial k_m} \\ \frac{\partial C_{2t}}{\partial k_1} & \frac{\partial C_{2t}}{\partial k_2} & \dots & \frac{\partial C_{2t}}{\partial k_m} \\ \dots & \dots & \dots & \dots \\ \frac{\partial C_{nt}}{\partial k_1} & \frac{\partial C_{nt}}{\partial k_2} & \dots & \frac{\partial C_{nt}}{\partial k_m} \end{bmatrix}, X_{(nq \times m)} = \begin{bmatrix} \frac{\partial \tau_{1q}}{\partial k_1} & \frac{\partial \tau_{1q}}{\partial k_2} & \dots & \frac{\partial \tau_{1q}}{\partial k_m} \\ \frac{\partial \tau_{2q}}{\partial k_1} & \frac{\partial \tau_{2q}}{\partial k_2} & \dots & \frac{\partial \tau_{2q}}{\partial k_m} \\ \dots & \dots & \dots & \dots \\ \frac{\partial \tau_{nq}}{\partial k_1} & \frac{\partial \tau_{nq}}{\partial k_2} & \dots & \frac{\partial \tau_{nq}}{\partial k_m} \end{bmatrix} \tag{4}$$

where

n : number of observation wells

m : number of pilot point locations

τ_{nq} : q^{th} arrival time quantile at observation well n

$$Q(\tau) = \frac{\int_0^\tau C(t)dt}{\int_0^\infty C(t)dt} \tag{5}$$

where

C : concentration

t : time

τ_q : q^{th} arrival time quantile when $Q(\tau_q) = q$

$Q(\tau)$: normalized cumulative concentration

For the second objective of hydraulic conductivity estimation, the optimal combination of different information sets in terms of minimal residual between measured and calculated data is applied using binary coefficient (i.e. 1 or 0). Relative comparisons are adopted to combine various data sets together.

$$\begin{aligned} MINIMIZE \text{ obj_2} = & W_a \sum_i \left(\frac{h_i^{obs} - h_i^{cal}}{h_i^{obs}} \right)^2 + \\ & W_b \sum_i \sum_t \left(\frac{C_{it}^{obs} - C_{it}^{cal}}{C_{it}^{obs}} \right)^2 + W_c \sum_i \sum_q \left(\frac{\tau_{iq}^{obs} - \tau_{iq}^{cal}}{\tau_{iq}^{obs}} \right)^2 \end{aligned} \tag{6}$$

where,

$W_{a,b,or c}$: Weighting coefficient

$h^{obs}, C^{obs}, \tau^{obs}$: Observed hydraulic head, tracer concentration, and tracer arrival quantile

$h^{cal}, C^{cal}, \tau^{cal}$: Calculated hydraulic head, tracer concentration, and tracer arrival quantile

4. Numerical Examples of DBM

4.1. Hypothetical Models

For synthetic model (i.e. true values), we generate a heterogeneous, confined, and isotropic site (1000 m by 700 m) using the kriging method as a tool of interpolation. In the kriging process, the exponential model ($\gamma(h) = c_0 + c_1 \cdot (1 - e^{-3|h|/a})$); where c_1 is sill, a is range, c_0 is nugget, and h is distance) is applied to generate heterogeneous hydraulic conductivity distribution. In this study sill (c_1) and range (a) values were 0.8619 and 3.4026, respectively without a nugget effect. The synthetic hydraulic conductivity was distributed between 5 and 85 mm/second. The flow field based on the synthetically generated hydraulic conductivity distribution has two Dirichlet boundaries and Neuman boundaries at leftmost and rightmost sides, and top and bottom sides of the synthetic site, respectively. For Dirichlet boundaries 20 m (leftmost side) and 15 m (rightmost side) are constantly applied without changes. In the flow figure, lower hydraulic conductivity location has significant flow bump, other than that mostly groundwater flow does not have vital changes in the given domain based on flow direction.

As previously mentioned, boundary conditions are imported from the flow code and pass the groundwater velocity profile to the advection terms in the contaminant transport code. In this synthetic transport code, we are applying instant a small line source as a tracer injection at the line shown in the Fig. 3 as a thick black line. We are assuming that we already know other information such as longitudinal and transverse dispersivities (100, and 20), molecular

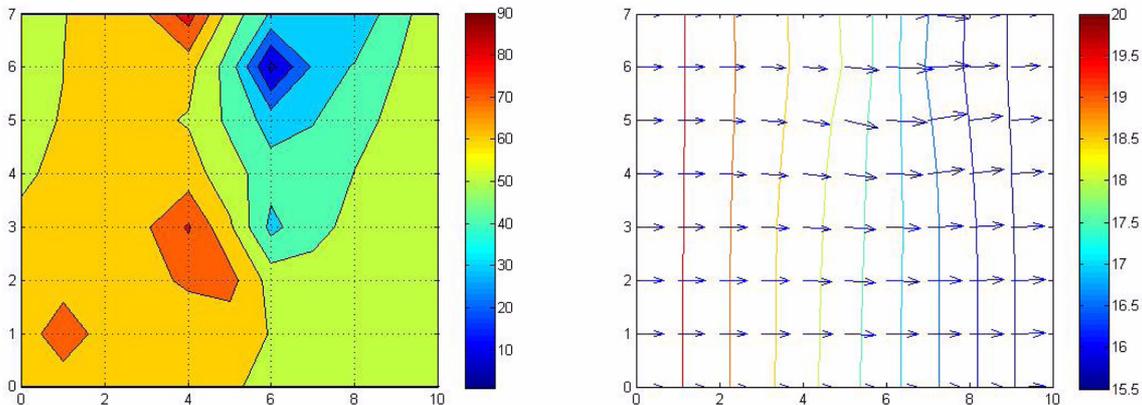


Fig. 2. Hydraulic conductivity distribution and groundwater flow for the true K-field.

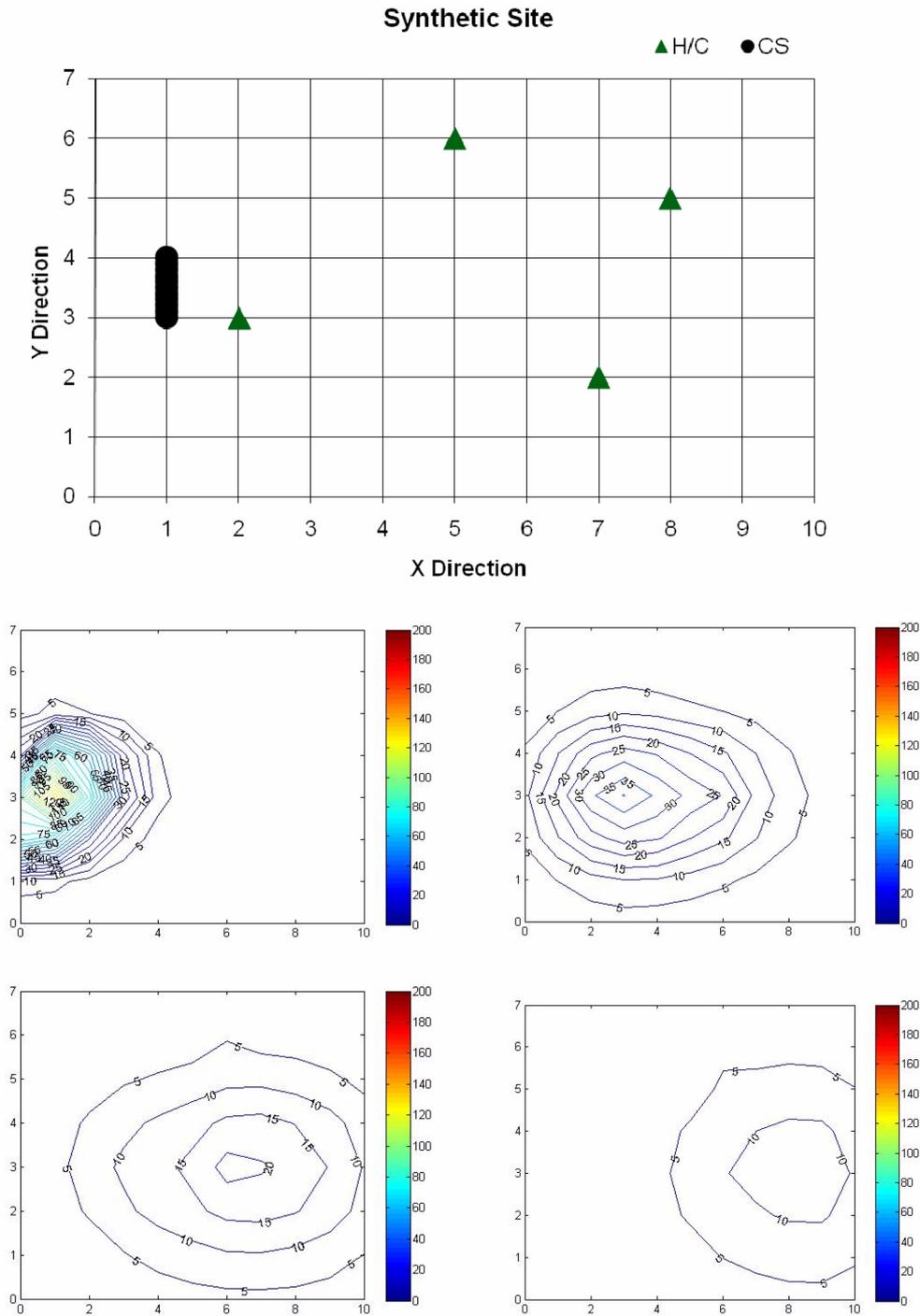


Fig. 3. Initial tracer injection line with the transport depending on time in the domain (triangles: observation points, and dark line: Tracer injection points in initial injection domain).

dispersion coefficient (0.01), retardation factor (1), and concentration of injected tracer (200 PPM) and injection period (2 days). The time period for concentration measurements at selected observation locations; the same locations as hydraulic head observations, is every 10 days period from

400 days generation and Fig. 3 shows the 20, 100, 200 and 300 day concentration contours (from left upper corner to right bottom corner) after injection using the hydraulic conductivity distribution of the true K-field.

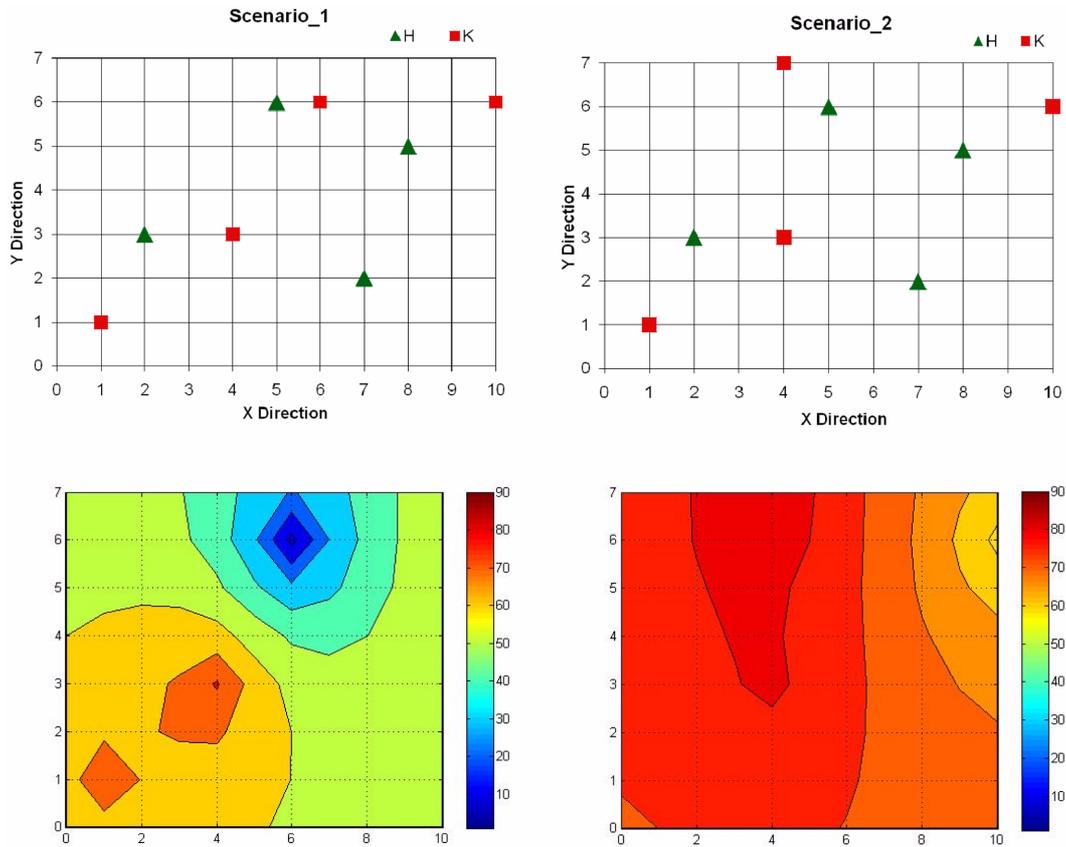


Fig. 4. Hydraulic conductivity measurement locations and initial hydraulic conductivity distributions based on kriged measurements for scenario 1 and 2.

4.2. Different Scenarios

Based on the true values of the hydraulic conductivity distribution, we generate two different hydraulic conductivity distributions, scenario 1 and 2. Four different locations are selected from true values as hydraulic conductivity measurements, and based on them initial conditions are generated by ordinary kriging function. Each scenario has four hydraulic head and/or concentration observation locations and does not change locations with time. Scenario 1 has closer pattern of hydraulic conductivity distribution to the true values than scenario 2, since scenario 2 does not include the highly significant point (6, 6) as a hydraulic conductivity measurement. Fig. 4 shows the locations of hydraulic conductivity measurements and observations, and initial hydraulic conductivity distributions of scenario 1 and 2.

4.3. Numerical Comparison

The purpose of parameter estimation in this study is to

obtain the optimal hydraulic conductivity distribution close to the true values based on several measurements. Therefore, if we want to see the performance of different combinations of various information sets in DBM, comparison between calculated and observed (in this study true K-field) hydraulic conductivities is a good indicator. As for numerical indicators, two different terms: average of square root hydraulic conductivity difference (K_{diff}) and maximum hydraulic conductivity differences ($MaxeK$) for whole grid points are shown in equations (7) and (8).

$$K_{diff} = \frac{1}{N_t} \sqrt{\sum_{i=1}^{N_t} (K_{ci} - K_{ri})^2} \quad (7)$$

$$MaxeK = \text{Max} [|K_{ci} - K_{ri}|], \quad \forall i \quad (8)$$

Where K_{ci} and K_{ri} are calculated and real (true) hydraulic conductivity at grid point i , and N_t is total number of grid points.

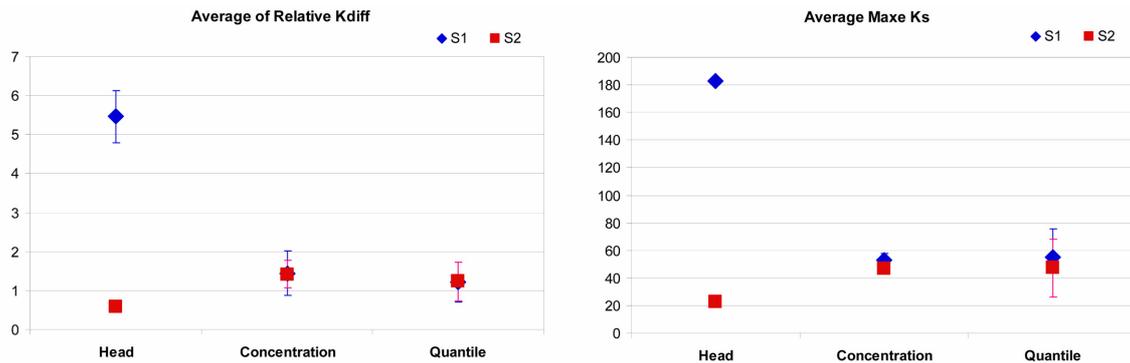


Fig. 5. Applications of D-optimality based pilot point methods using individual information set separately.

Table 1. Coupled inverse problems in different conditions (W_a , W_b , W_c : Weights for hydraulic heads, tracer concentration, and tracer arrival quantile, respectively)

Scenario	PPL Selection	Weight			Average		Standard Deviation	
		W_a	W_b	W_c	K_{diff}	$MaxeK$	K_{diff}	$MaxeK$
S_1	Head Only	1	0	0	5.46435	182.168	0.664628	23.09931
	Head & Con	1	1	0	0.618474	31.4524	0.025131	1.259951
	Head & Quan	1	0	1	1.944652	75.4416	1.725637	69.17361
	Head & Con & Quan	1	1	1	1.33119	54.2901	0.423429	21.11354
S_2	Head Only	1	0	0	0.579468	23.0113	0.084991	0.82505
	Head & Con	1	1	0	0.604876	24.1362	0.183736	5.112352
	Head & Quan	1	0	1	1.589397	53.8188	0.948019	36.9898
	Head & Con & Quan	1	1	1	11.0705	355.5363	7.971674	241.2799

5. Results and Discussions

Before we tried to see the performance of coupled inverse problems, we carried out ten different trials of DBM with individual information sets (i.e. hydraulic head, tracer concentrations, and tracer arrival time quantiles). Fig. 5 gives the results of average of average square root hydraulic conductivity difference (K_{diff}) and average of maximum differences ($MaxeK$) between calculated and true hydraulic conductivities. Averages of K_{diff} and $MaxeK$ show the similar pattern in different data applications for both scenario 1 and 2. For scenario 1 when only head information is used for optimizing pilot point locations and hydraulic conductivities at those selected locations, average of K_{diff} was higher than when other information is used. Alternatively, for scenario 2 when only head information is used, lower averages of K_{diff} and $MaxeK$ are found. Using only concentration data gives significant reduction in variance of

hydraulic conductivity solutions for scenario 1, however in scenario 2, the variance increased with only concentration data application. Furthermore, tracer arrival time quantile produces even higher fluctuations in the hydraulic conductivity search. In scenario 2 when only head information is used, the significant pilot point location (6, 6), which has lowest hydraulic conductivity value, was selected in the pilot point location search. Therefore the final hydraulic conductivity values much closer to the true K-field and gives less variance of hydraulic conductivity searches. When we use coupled inverse problems for hydraulic conductivity estimation, pilot point locations from either head only or concentration only can be used to find optimal hydraulic conductivity distribution. For the second procedure of DBM, using only one information set is not recommended for hydraulic conductivity search, when we have different kinds of information available.

For the comparison of coupled inverse problems, three

different pilot point location sets based on the applications of diverse information sets (i.e. head only, head and concentration, and head and quantile) are considered. For scenario2, two different pilot point location sets were tested because optimized pilot point locations using hydraulic head or concentration are in very similar positions in the domain. Using the selected pilot point locations, different combinations of information are used to get hydraulic conductivity values. As a conclusive procedure of this study, firstly we only used hydraulic head data and then combined hydraulic head data with tracer concentrations for hydraulic conductivity search. Finally hydraulic head data with tracer arrival time quantile was used for both pilot point locations from only head and the combination from head and quantiles for both pilot point locations and hydraulic conductivity searches. Table 1 presents the final results of averages of K_{diff} and $MaxeK$ with diverse combinations. When hydraulic head and tracer arrival quantiles are used for head only pilot point locations, the averages of K_{diff} and $MaxeK$ are increased and also the variances of both numerical criteria follow a similar pattern. Furthermore for combined locations based on heads and quantiles, K_{diff} and $MaxeK$ are getting significantly worse with increased variances for both scenarios. The reason for this is possibly that the combined pilot point locations introduce the different covariance structure and gives more uncertainties in the hydraulic conductivity search procedure. However, the coupled inverse problem of hydraulic head and tracer concentration for hydraulic conductivity search is promising the robustness of hydraulic conductivity estimation either initial hydraulic conductivity close to the true K-field or not. Clearly, in scenario 1, averages of K_{diff} and $MaxeK$ are significantly decreased with minimized variances for both, and in scenario 2, both numerical criteria are close to the best result from head only. When both information of head and concentration is used for both procedure with bigger population size for GA search (10000 shown in Table 1), the accuracy of searching hydraulic conductivity is increased in scenario 1. Based on these results, selection of pilot point locations based only on hydraulic heads and fitting of hydraulic conductivities from hydraulic heads and tracer concentrations are recommended in groundwater flow parameter identification.

6. Conclusion

Several techniques have been investigated to improve various aspects of subsurface parameter identification over the recent decades. One method to significantly improve the techniques is the application of coupled inverse problems. In D-optimality based pilot point method (DBM), this coupled inverse problem is applied and compared with diverse conditions. For the first step of the pilot point locations search, three different information sets (e.g. hydraulic heads, tracer concentrations, and tracer arrival time quantiles) are individually applied and compared to each other. In this comparison, pilot point locations based on hydraulic heads has similar trends to the pilot point locations from tracer concentrations. Thus hydraulic head based pilot point locations and the combination of each head and quantiles respectively based pilot point locations were selected and applied to the second procedure comparisons. In the second step, the procedure of hydraulic conductivity search was based on the combination of various information sets in order to give an idea of the optimal set of information based on given pilot point locations. In the results, a set of measurements of hydraulic heads and tracer concentrations was a promising tool for hydraulic conductivity search with the pilot point locations based on heads for either initial hydraulic conductivities close to the true K-field or not. In this study, plausibility terms are not applied to constrain the hydraulic conductivity search space to make it more similar to the true K-field, because tracer concentration information inherently provide the magnitude of hydraulic conductivity while hydraulic heads gives the trends of hydraulic conductivity distributions. Tracer arrival time quantiles with hydraulic heads for pilot point locations and hydraulic conductivity search introduced more uncertainties in our case.

References

- de Marsily, G., Lavedan, G., Boucher, M., and Fasanino, G., 1984, Interpretation of interference tests in a well field using geostatistical techniques to fit the permeability distribution in a reservoir model, Geostatistics for natural resources characterization, pt.2, D. Reidel Publishing Company.
- Franssen, H.H., Hernandez, J.G., and Sahuquillo, A., 2003, Coupled inverse modeling of groundwater flow and transport and

- the worth of concentration data, *J. Hydrol.*, **281**, 281-295.
- Gomez-Hernandez, J.J., Sahuquillo, A., and Capilla, Jose E., 1997, Stochastic simulation of transmissivity fields conditional to both transmissivity and piezometric data-I. Theory, *J. Hydrol.*, **203**, 162-174.
- Harvey, C.F. and Gorelick, S.M., 1995, Mapping hydraulic conductivity: Sequential conditioning with measurements of solute arrival time, hydraulic head, and local conductivity, *Water Resour. Res.*, **31**(7), 1615-1626.
- Jung, Y., Ranjithan, R.S., and Mahinthakumar, G., 2011, Subsurface characterization using a D-optimality based pilot point method, *J. Hydroinform.*, **13**(4), 775-792.
- Knopman, D.S. and Voss, C.I., 1987, Behavior of sensitivities in the one-dimensional advection-dispersion equation: Implications for parameter estimation and sampling design. *Water Resour. Res.*, **23**(2), 253-272.
- Kowalsky, M.B., Finsterle, S., Williams, K.H., Murray, C., Commer, M., Newcomer, D., Englert, A., Steefel, C.I., and Hubbard, S.S., 2012, On parameterization of the inverse problem for estimating aquifer properties using tracer data, *Water Resour. Res.*, **48** (W06535), 1-25.
- LaVenue, A.M. and Pickens, J.F., 1992, Application of a coupled adjoint sensitivity and kriging approach to calibrate a groundwater flow model, *Water Resour. Res.*, **28**(6), 1543-1569.
- LaVenue, A.M., RamaRao, B.S., de Marsily, G., and Marietta, M.G., 1995, Pilot point methodology for automated calibration of an ensemble of conditionally simulated transmissivity fields Application, *Water Resour. Res.*, **31**(3), 495-516.
- Liang, L., Zeng, G., Guo, S., Wei, A., Li, X., Shi, L., and Du, C., 2010, Optimal solute transport in heterogeneous aquifer: Coupled inverse modeling, *Inter. J. Environ. Pollu.*, **42**(1-3), 258-269.
- Mishra, S. and Parker, J.C., 1989, Parameter estimation for coupled unsaturated flow and transport, *Water Resour. Res.*, **25**(3), 385-396.
- Medina, A., Carrera, J., and Galarza, G., 1990, Inverse modeling of coupled flow and solute transport problem, ModelCARE 90: Calibration and Reliability in Groundwater Modeling, IAHS Publ. 195.
- RamaRao, B.S., LaVenue, A.M., de Marsily, G., and Marietta, M.G., 1995, Pilot point methodology for automated calibration of an ensemble of conditionally simulated transmissivity fields 1. Theory and computational experiments, *Water Resour. Res.*, **31**(3), 457-493.
- Strecker, E.W. and Chu, W.-S., 1986, Parameter identification of a ground-water contaminant transport model, *Ground Water*, **24**(1), 56-62.
- Sun, N.-Z. and Yeh, W.W.-G., 1990, Coupled inverse problems in groundwater modeling, 1. Sensitivity analysis and parameter identification, *Water Resour. Res.*, **26**(10), 2507-2525.
- Van Rooy, D., Keidser, A., and Rosbjerg, D., 1989, Inverse modeling of flow and transport, Groundwater Contamination, Third IAHS Scientific Assembly, IAHS Publ. No. 185.
- Wen, X.-H., Deutsch, C.V., and Cullick, A.S., 2002, Construction of geostatistical aquifer models integrating dynamic flow and tracer data using inverse technique, *J. Hydrol.*, **255**, 151-168.
- Woodbury, A.D., Smith, L., and Dunbar, W.S., 1987, Simultaneous inversion of hydrogeologic and thermal data, 1, Theory and application using hydraulic heat data, *Water Resour. Res.*, **23**(8), 1586-1606.
- Woodbury, A.D. and Smith, L., 1988, Simultaneous inversion of hydrogeologic and thermal data, 2, incorporation of thermal data, *Water Resour. Res.*, **24**(3), 356-372.